

USER: Brian.Miller

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ATIC CONC ESP BIG RE													S				Sheet No. Description S1 Title Sheet S2 General Notes S3 Layout S4 Concrete Removal Details S5 Intermediate Bent and End Bents S6 Box Beam General Notes S7 Box Beam CB17 Details S8 Box Beam CB33 Details S9 Superstructure S10-S11 Construction Elevations S12 Detour S12 Detour S12 Detour S12 Detour Special Note for Concrete Sealing Special Note for Embedded Galvanic Anodes Type 1A
ESI											IE	25	00044	0011150	00000	0074450	
BID ITEM	Concrete Class "A"	Concrete Class "AA"	Concrete Sealing	Masonry Coating	Steel Reinforcement, Epoxy Coated	Remove Superstructure	PPC Box Beams CB-17	PPC Box Beams CB-33	Rail System Side Mounted MGS	Armored Edge for Concrete	Remove & Reset Guardrail	Edge Key	Barricade Type III	Galvanic Anode- XPX	Foundation	Epoxy Injection Crack Repair	BBP-003-02 Elastomeric Bearing Pads for Box Beams BGX-006-10 Stencils for Structures
UNIT End Bent #1	C.Y. 1.6	C.Y.	S.F.	S.Y. 11	LBS. 177	L.S.	L.F.	L.F.	L.F.	L.F.	L.F.	L.F.	Each	Each 50	L.S.	L.F. 31	BGX-012-02 Geotechnical Legend BJE-001-14 Nepprene Expansion Dams and Armored Edges
End Bent #2	1.6 1.9			11 118	177 258									50 50		20 31	BGX-022 Joint Waterproofing BHS-011 Railing System Side Mounted MGS Details
Superstructure		48.6	3126		6846		264	462	237.5	55.4							BDP-001-06 Replaced by sheet S6 BDP-002-03 Box Beam Bearing Details
BRIDGE TOTALS	5.1	48.6	3126	140	7458	1	264	462	237.5	55.4	100	48	2	150	1	82	BDP-003-03 Box Beam Miscellaneous Details
BID ITEM CODE	02726	02650	02569	02568	00001	00214	00339	02562	00356								BDP-010-04 Box Bealth Tension Rod Details BDP-010-04 Replaced by sheet S8
BID ITEM	Staking	aintain and ntrol Traffic	mobilization	Mobilization- For Concrete Sealing	DGA Base	:L3 ASPH BASE 1.00D PG64-22	2L3 ASPH SURF 0.38D PG64-22	Temporary Signs	Asphalt Material for Tack								BDP-007-05 Replaced by sheet S7
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UNIT End Bent #1 End Bent #2 Intermediate Bent Superstructure	L.S.			L.S.	Ton	Ton	Ton	S.F.	Ton								- - - - <t< td=""></t<>
UNIT End Bent #1 End Bent #2 Intermediate Bent Superstructure BRIDGE TOTALS	L.S.	ŬÖ L.S.	e L.S.	L.S.	Ton	59	Ton 44	S.F.	Ton								
UNIT End Bent #1 End Bent #2 Intermediate Bent Superstructure BRIDGE TOTALS	L.S.	ŬÖ L.S.	e L.S.	L.S.	Ton	59	Ton	S.F.	Ton								Image: Second system
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DATE PLOTTED: 23-OCT-2024							

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	UNIT End Bent #1	C.Y. 1.6	C.Y.	S.F.	S.Y. 11	LBS. 177	L.S.	L.F.	L.F.	L.F.	L.F.	L.F.	L.F.	Each	Each 50	L.S.	L.F. 31	BGX-01 BJE-001	2-02 Geo -14 Nepp	technical L prene Expa	egend Insion Dams and Armored	Edges	
	End Bent #2	1.6 1.9			11 118	177 258									50 50		20 31	BGX-02 BHS-01	2 Joint V 1 Railing	Waterproof g System S	fing Side Mounted MGS Details		
	Superstructure		48.6	3126		6846		264	462	237.5	55.4							BDP-00	1-06 Repl 2-03 Box	laced by sh Beam Bea	neet S6 ring Details		
	BRIDGE TOTALS	5.1	48.6	3126	140	7458	1	264	462	237.5	55.4	100	48	2	150	1	82	BDP-00	3-03 Box	Beam Misc	cellaneous Details		
	BID ITEM CODE	02726	02650	02569	02568	00001	00214	00339	02562	00356								BDP-00	0-04 Repl	laced by sh	neet S8		
	BID ITEM	Staking	Maintain and Control Traffic	Demobilization	Mobilization- For Concrete Sealing	DGA Base	CL3 ASPH BASE 1.00D PG64-22	CL3 ASPH SURF 0.38D PG64-22	Temporary Signs	Asphalt Material for Tack								BDP-00	8 Guarc	laced by sh	n Transition neet S7		
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	PREPARED BY		DATE: J	ULY 2024			CH		ſ		Т	ITLE	SHE	ET					ROU	ΤE	BRIDGE ID. 031B00027N		N
Stri	uctural Design	L	DESIGNE	DBY: J.	BAWITH	AWNG	J. VAN	NDARDS				BIG		ROSSING	EFK				KY 23	38	SHEET NO.	DRAWING NUMBEI	R
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SPECIFICATIONS: All references to the Specifications are to the current edition of the Kentucky CONCRETE SEALER: All areas detailed in the specifications as requiring masonry coating shall Department of Highways Standard Specifications for Road and Bridge Construction with current sealed in accordance with the special note for concrete sealing. The superstructure deck, barrier and overhangs shall also be sealed as shown herein these plans. Concrete surfaces (except t deck) shall receive the ordinary surface finish as described in section 601.03.18(A) prior to beir sealed. DESIGN LOAD: This bridge is designed for a KYHL-93 live load. The KYHL-93 live load is arrived at CONCRETE REMOVAL: Any concrete removal outside the detailed limits shall be replaced by increasing the standard HL-93 truck and lane loads as specified in the AASHTO Specifications by at the contractor's expense. The contractor shall make a saw cut at the removal limits to 25%. form a neat construction joint. All costs for the saw cut & concrete removal are included in the price bid for, "Remove Existing Superstructure". FUTURE WEARING SURFACE: This structure is designed for a 15 PSF future wearing surface load. DAMAGE TO THE SUBSTRUCTURE: The contractor is responsible for any and all damages to t existing substructures during reconstruction even to the replacement of the entire substructure, should they be damaged due to his actions. ON-SITE INSPECTION: Each contractor submitting a bid for this work shall make a thorough inspection of the project site prior to submitting a bid and shall be thoroughly familiarized with existing conditions so that work can be expeditiously performed after a contract is awarded DESIGN METHOD: All reinforced concrete members are designed by the load and resistance factor Submission of a bid will be considered evidence of this inspection having been made. Any claim method as specified in the current AASHTO Specifications. resulting from site conditions will not be honored by the Department of Highways.

Supplemental Specifications. All references to the AASHTO Specifications are to the current edition of the AASHTO LRFD Bridge Design Specs, with interims.

DESIGN STRESSES:	Concrete Class "A"	~	f'c = 3500 psi
	Concrete Class "AA"	~	f'c = 4000 psi
	Steel Reinforcement	~	Fy = 60,000 ps
	Structural Steel Yield Strength	~	Fy = 50,000 ps

WIND LOAD: This bridge is designed for a wind load based on a wind velocity of 100 mph.

REINFORCEMENT: Dimensions shown from the face of concrete to bars are to center of bars unless otherwise shown. Spacing of bars is from center to center of bars. Clear distance to face of concrete is 2", unless otherwise noted. Any reinforcement bars designed be suffix (e) in the plans shall be epoxy coated in accordance with section 811.10 of the Standard Specifications. Any reinforcing bars designated by suffix (s) in a bill of reinforcement shall be considered a stirrup for purposes of bend diameters.

BEVELED EDGES: Bevel all exposed edges $\frac{3}{4}$ " unless otherwise noted.

COMPLETION OF THE STRUCTURE: The Contractor is required to complete the structure in accordance with the plans and specifications. Material, labor or construction operations, not otherwise specified, are to be included in the bid item most appropriate to the work involved. This may include cofferdams, shoring, excavations, backfilling, removal of all or parts of existing structures, phase construction, incidental materials, labor or anything else required to complete the structure.

SHOP DRAWINGS: Submit shop drawings that are required by the plans and specifications directly to the Division of Structural Design. Is any changes in the design plans are proposed by a fabricator or supplier, submit those changes to the Department through the Contractor.

MASONRY COATING: Apply masonry coating to substructures in accordance with the specifications. Superstructure shall recieve concrete sealing in accordance with the Special Note for Concrete Sealing.

SUPERSTRUCTURE SLAB: Ensure the entire superstructure slab is poured continuously, out to out, before allowing any concrete to set.

DIMENSIONS: Dimensions are for a normal temperature of 60 degrees Fahrenheit. Layout dimensions are horizontal dimensions.

CONCRETE: Class "AA" Concrete is to be used throughout the superstructure. Class "A" concrete is to be used in the substructures. Prestressed beam concrete shall be in accordance with the plans and specifications.

REMOVAL OF EXISTING SUPERSTRUCTURE: The existing superstructure shall be removed in accordance with section 203 of the Specifications, except all materials of the superstructure shall become property of the contractor and shall be removed from the right-of-way. Care shall be taken to prevent damage to the substructure during this procedure. The cost of removing the superstructure shall be included in the lump sum bid for "Remove Existing" Superstructure".

CORK/STYROFOAM: The cost for the cork/styrofoam under and cork up the sides of the slab and abutment wings is incidental to the unit price bid for Class "AA" Concrete.



COMMONWEALTH OF KENTUCKY DEPARTMENT OF HIGHWAYS

REVISION

GENERAL NOTES

FIELD MEASUREMENTS: Prior to beginning work or ordering any materials, the contractor shall verify all dimensions and elevations. No claim shall be honored by the Department of Highways regarding site conditions.

ELEVATIONS: The contractor is to verify existing elevations and make a reference benchmark before removing any concrete.

EXISTING STRUCTURE: See drawing number 20922 and std. A410 for details of existing structure.

EXISTING REINFORCING STEEL: Take care not to damage existing reinforcement during concrete removal. Bend, straighten, abrasive blast clean, and coat with cold galvanizing material prior to adding zinc anodes. All costs to do this work is incidental to Class A Concrete.

DATE	PREPARED BY	DATE: JULY 2024	CHECKED BY		ROUTE	BRIDGE ID.	COUNTY OF
10/21/24	Division of			GENERAL NUTES		031B00027N	EDMONSON
		DESIGNED BY: J. VAN ZEE	S.T. ANDARDS	CROSSING	KV 220		
	Structural Design		U VAN ZEE		NT 230	SALET NO.	28916
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The following abbreviations may have been used in the preparation of these plans:

bet.	between
b.f.	Back Face
BOF	Bottom of Footing
BOS	Bottom of Slab
bot.	Bottom
Brg.	Bearing
C to C	Center to Center
c.e.	Current Edition
C.Y.	Cubic Yards
Chd.	Chord
CL	Center Line
Clr.	Clear
Conc.	Concrete
Cu.	Cubic
Dwg.	Drawing
e.f.	Each Face
EI.	Elevation
eq.	Equal
Est.	Estimate
Ext.	Exterior
F to F	Face to Face
f.f.	Front Face
f.s.	Far Side
fr.	Front
ft.	Feet
I.D.	Inside Diameter
in.	Inch
Int.	Interior
L	Left
LBS	Low Bridge Seat
LBS.	Pounds
Μ	Meter
MPH	Miles Per Hour
n.s.	Near Side
O.D.	Outside Diameter
Opp.	Opposite
PC	Point of Curvature
Perp.	Perpendicular
PI	Point of Intersection
PPC	Precast Prestressed Concrete
PPCDU	Precast Prestressed Deck Unit
PSI	Pounds per Square Inch
PT	Point of Tangency
R	Radius
R	Right
RCBC	Reinforced Concrete Box Culvert
RCDG	Reinforced Concrete Deck Girde
Req'd	Required
RR	Railroad
Shld.	Shoulder
spa.	Spaces
Sta.	Station
Std.	Standard
Str.	Straight
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DATE	PREPARED BY	DATE: JULY 2024	CHECKED BY	
	Division of			
		DESIGNED BT. J. VAN ZEE	S.I. ANDARDS	
	Structural Design	DETAILED BY: M. BAWITHAWNG	J. VAN ZEE	



USER: Brian.Miller

DATE	PREPARED BY	DATE: JULY 2024	CHECKED BY	CON
	Division of	DESIGNED BY: J. VAN ZEE	S.T. ANDARDS	
	Structural Design	DETAILED BY: M. BAWITHAWNG	J. VAN ZEE	



USER: Brian.Miller

			DESIGNED BY: J. VAN ZEE	S.I. ANDARDS	_
		Structural Design	DETAILED BY: M. BAWITHAWNG	J. VAN ZEE	
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PRECAST PRESTRESSED BOX BEAMS

SPECIFICATIONS: All references to the standard Specifications are to the current edition of the Kentucky Department of Highways Standard Specifications for Road and Bridge Construction, with current supplemental specifications. All references to the AASHTO Specifications are to the current edition of the AASHTO LRFD Bridge Design Specifications, with interims.

DESIGN LOADS: Beam sections are designed for 1.25*HL93 (KYHL93) Live Load.

DESIGN LOAD DISTRIBUTION: Contrary to AASHTO LRFD Bridge Design Specifications, the design moment and shear distribution for all beams is 0.5 lanes

FUTURE WEARING SURFACE: These beams are designed for a 15 PSF future wearing surface load.

SUBSTRUCTURE DESIGN LOADS: Unfactored design reaction forces per beam end. DC (kips): Beam, Slab (if applicable), and Type II railing dead loads. DW (kips): Future wearing surface. LL (kips): Beam Live Load reaction per lane x Design load distribution.

LL+I (kips): LL with Dynamic load allowance.

DESIGN DEFLECTIONS:

 \Box d (in.): Sum of the downwards deflections caused by the design 5" deck, railing, and future wearing surface. (Positive Downwards)

□ c (in.): Upwards midspan camber of the beam caused by prestressing minus the downward deflection of the beam due to self weight. (Positive Upwards)

MATERIAL DESIGN SPECIFICATIONS: for Steel Reinforcement for Prestressed Girder Concrete (Typ. U.N.O.)

for Class "AA" Concrete for Prestressing Steel

DESIGN LENGTH: Beam lengths shown in the Standards represent total beam length. Use the next areater designed section for non-Standard lengths.

CONSTRUCTION METHOD: Transferring bond stress to the concrete will not be allowed, nor releasing of end anchors until the concrete has attained a minimum compressive strength of F'CI as shown by standard cylinders made and cured identically with the girders; attain F'C at or prior to 28 days. Apply an initial prestress force of 33817 lbs. per low relaxation strand. Beams with honeycomb of such extent as to affect the strength of resistance to deterioration will not be accepted. The allowance of 0005L (length) is made for shortening of beams due to shrinkage and elastic change. Furnish shop plans showing a detensioning plan by numbering, in sequence, the strand pattern.

PRESTRESSING STRANDS: Ensure prestressing strands to be b" oversize (0.167 sq. in.) uncoated sevenwire stress relieved, low-relaxation strands conforming to AASHTO M 203, Grade 270. If an alternate strand arrangement or strand type is preferred by the Contractor, the designer that developed the original plans will provide the design and also revise the original plans to reflect the changes. These design and plan modifications will be done at the Contractor's expense.

CORROSION INHIBITOR: Provide a corrosion inhibitor for B-type (non-composite) beams from the list of approved materials.

BEVELED EDGES: Bevel all exposed edges 5".

BEAM SEALER: For composite box beams (CB Beams), seal the full length of the exterior face of all exterior beams with the extent from the top of the beam to 1'- 0" underneath the beam. For noncomposite box beams (B beams), seal all faces of all beams, except take care to ensure the grout pockets are not sealed. Use an approved silane sealer as specified by the Division of Structural Design.



COMMONWEALTH OF KENTUCKY DEPARTMENT OF HIGHWAYS

REVISION

General Notes

FY = 60000 PSIF'C = 7000 PSIF'CI = 5500 PSI F'C = 4000 PSIF'S = 270000 PSI

REINFORCEMENT: Dimensions shown from the face of concrete to reinforcement are clear distances. Spacing of reinforcement is from center to center of reinforcement. All steel reinforcement is to be epoxy coated in accordance with Section 811.10 of the Specifications. Consider bars marked "C" to be a stirrup for purposes of bend diameters. Non-epoxy reinforcement may be used for fabrication purposes, only, provided that the steel is not used in the top 5 $\frac{1}{2}$ " of the beam and the location of the steel is indicated on the shop drawings.

FABRICATION: Beams shall not be fabricated more than 120 days before the deck is to be poured.

GROUT: Provide non-shrink grout for anchor dowels, shear keys, and tensioning rod blockouts conforming with Section 601.03.03 of the Specifications. When side by side superstructure is utilized, grouting will be completed after lateral tension rods have been fully tightened and before leveling devices have been removed. Include the cost of furnishing and placing grout in the price of beam.

RAILING SYS	STEM TYPE II:	Furnish this material per these specifications.	
ITEM	DESCRIPTION	MATERIAL SPECIFICATION	COATING SPECIFICATION
Post	W6x25	ASTM A36 or A572	A123
Channel	C7x9.8	ASTM A36 or A572	A123
Plate	¹ ⁄2"x 7"	ASTM A36 or A572	A123
Tubing	8x4x0.1875	ASTM A500 or A501	A123
Bolts	5/ ₈ "	ASTM A307	A153
Nuts	for ⁵ / ₈ "	ASTM A563, Grade A or better	A153
Washers	for ⁵ ⁄8"	ASTM A563, Grade A or better	A153
Stud	1 ¹ ⁄ ₄ "	ASTM A108 (1045 C.D. Bar)	B633, Type II, Class 25
Ferrule	2 ¹ ⁄ ₂ "x 5"	ASTM A108 (11L17 Steel)	B633, Type II, Class 25
Wire	3/8"	ASTM A510 (1018 Steel)	B633, Type II, Class 25
Nut	for 1 $\frac{1}{4}$ " Bolt	ASTM A108 (12L14 Steel)	B633, Type 1, Class 25
Nut	for 1 $\frac{1}{4}$ " Stud	ASTM A325M	B633, Type II, Class 25
Washers	for 1 $\frac{1}{4}$ " Stud	ASTM A325M	B633, Type II, Class 25

RAILING	SYSTEM	SIDE	MOUNTED	MGS:	ls	to	b
011, c.e.							

Use the current edition of the references listed below with these standards.
STANDARD DRAWINGS
BBP-003 Elastomeric Bearing Pads
BHS-007 Railing System Type II
BJE-001 Armored Edge & Neoprene Joints
RBR-001 Steel Beam Guardrail
RBR-005 Guardrail Components
SPECIAL NOTES

for Corrosion Inhibitors

PREPARED BY DATE DATE: JULY 2024 CHECKED BY BO Division of DESIGNED BY: J. VAN ZEE S.T. ANDARDS Structural Design DETAILED BY: M. BAWITHAWNG J. VAN ZEE

DATE PLOTTED: 23-OCT-2024

е	used	on	this	structure.	see	Std.	Dwa.	BHS-
	uscu		uno	Structure,	000	Old.	Ding.	DIIO

	DOUTE	BRIDGE ID.	COUNTY OF
X BEAM GENERAL NOTES	ROUTE	031B00027N	EDMONSON
CROSSING BIG REEDY CREEK	KY 238	SHEET NO. S6	drawing number 28916



USER: Brian.Miller

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$GBT = \underbrace{BT}_{(2)} \underbrace{BT}_{(2)$			□□Debond these△Debond these	strands 4' each end of beam strands 6' each end of beam	— CB17-48 B	eam Only		42 7	7" 5½" 10" 8" (11 ¹ 2" 14"			
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$\frac{1}{2} \underbrace{(1 \text{ bars } 0; 12 \text{ spaces, maximum}}_{2} (1 \text{ bars$							CB17	42 7 44 8	7" 8 ¹ / ₂ " 12" 7" 7 ¹ / ₂ " 12"			
$\frac{1}{2} + \frac{1}{2} + \frac{1}$								46 8 48 10	7" 7 ¹ / ₂ " 12" 6" 9 ¹ / ₂ " 12"			
$\frac{1}{2} \underbrace{1}_{1-2} \underbrace{1}_{2-2} \underbrace$							TABLE OF	BAR QUA	ANTITIES	ESIGN	DATA	
$\frac{1}{1} \underbrace{(1 + \frac{1}{2}, \frac{1}$			C1 bars @ 12" spaces, maximum 2 ¹ / ₂ " "F" spaces @ "G" "H",	m ► C4 bars @ "J"	~		Beam Type (feet)	n C1 C2 C	3 C4 DC (kips) (JWV LL LL <ips)< td=""> (kips) (kips)</ips)<>	+ □d □c ɔs) (in.) (in.)	
$\frac{c_{1}}{c_{2}} + \frac{c_{4}}{c_{4}} + \frac{c_{4}}{c$	►						B17	$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	9.4 10.1 10.7 11.4 12.1	37.8 35.1 44 0.9 36.4 46 0.9 37.7 48 1.0 38.9 46 1.0 40.0 50 1.1 41.1 51	9 .4 .4 .6 .6 .2	
1-C3-(38-38) 1 <t< th=""><th></th><th>1'-5"</th><th>$\begin{array}{c c} C1 \\ C4 \\$</th><th></th><th></th><th></th><th>40 42 36 38</th><th>81 53 85 56 37 39</th><th>13.4 14.1 37 16.6 39 17.5</th><th>1,1 42,1 53 1,2 43,8 54 1,0 40,0 50 1,1 41,1 52</th><th>.4 .5 .9 0.2 0.6 .2 0.6</th></t<>		1'-5"	$\begin{array}{c c} C1 \\ C4 \\$				40 42 36 38	81 53 85 56 37 39	13.4 14.1 37 16.6 39 17.5	1,1 42,1 53 1,2 43,8 54 1,0 40,0 50 1,1 41,1 52	.4 .5 .9 0.2 0.6 .2 0.6	
C2-03-(40-48') C2(e)-C4(e) Bars C2(e)-C4(e) Bars C2(e)-C4(e) Bars CB17 ELEVATION OF O° SKEW (Refer to BDP-003,c.e. for skewed details) Standard and an and an and and and and and and		1	~C3~(36'-38')			a	CB17	43	41 184	1.1 42.1 531.2 43.0 541.3 43.0 51	.4 0.2 0.8 .5 0.3 0.9	
CB17 ELEVATION OF O° SKEW (Refer to BDP-003,c.e. for skewed details) Straight Reinforcement Mark Size Length A(E) #4 Beam Length Minus 3" A2(E) #4 Beam Length Minus 3" A2(E) #4 Beam Length Minus 3" D(E) #8 2-0" Bent Reinforcement Mark Size a C1(e) Bar Reinforcement Mark Size a Date Division of Structural Design Date: JULY 2024 CHECKED BY CB17 BOX BEAM DETAILS Route BIOREID DATE Division of Structural Design Date: JULY 2024 CHECKED BY CHECKED BY CHECKED BY CHECKED BY CB17 BOX BEAM DETAILS ROUTE BIOREID COUNT OF 318000027N DEMEROUNDER DATE JULY 2024 CHECKED BY CHECKED BY COUNT OF <th colspa="</td"><td></td><td><u>↓</u> 2</td><td>-C3~(40'-48')</td><td></td><td></td><td>C2(e)-C4(e) Bars</td><td></td><td>47 49</td><td>54 21.1 56 22.0</td><td>1.3 44.7 56 1.4 45,5 57</td><td>.5 0.3 1.0 .5 0.3 1.3 .4 0.4 1.8</td></th>	<td></td> <td><u>↓</u> 2</td> <td>-C3~(40'-48')</td> <td></td> <td></td> <td>C2(e)-C4(e) Bars</td> <td></td> <td>47 49</td> <td>54 21.1 56 22.0</td> <td>1.3 44.7 56 1.4 45,5 57</td> <td>.5 0.3 1.0 .5 0.3 1.3 .4 0.4 1.8</td>		<u>↓</u> 2	-C3~(40'-48')			C2(e)-C4(e) Bars		47 49	54 21.1 56 22.0	1.3 44.7 56 1.4 45,5 57	.5 0.3 1.0 .5 0.3 1.3 .4 0.4 1.8
(Refer to BDP-003, c.e. for skewed details) A1(E) #5 Beam Length Minus 3" A2(E) #4 Beam Length Minus 3" D(E) #8 2-0" Bent Reinforcement Mark Size a b C1(e) #5 3'-9" 6" C1(e) #5 3'-9" 6" C1(e) #5 3'-9" 6" C1(e) #5 3'-9" 1.4 ½* C3(e) #4 3'-9" 1.4 ½* C3(e) #4 3'-9" 1.4 ½* C4(e) #4 3'-9" 1.4 ½* DATE JULY 2024 CHECKED BY CB17 BOX BEAM DETAILS ROUTE DIVISION OF Structural Design Design Design Structures ST. ANDARDS BIG REEDY CREEK DATE DIVISION OF Structures CHECKED BY J. VAN ZEE ST. ANDARDS BIG REEDY CREEK			CB17 ELEVATION OF O° SK	EW				Straigh Mark	t Reinfor	<u>Cement</u> Lenath	<u>.</u>	
DIE Wark Size a b			(Refer to BDP-003,c.e. for skewed detai	ls)				A1(E) A2(E)	#5 Be #4 Be	am Length M am Length N	linus 3" 1inus 3"	
Bent Reinforcement Mark Size a b C1(e) Bar C1(e) Bar C3(e) #44 3*9" 1*1*3%" Date Division of Structural Design Date: JULY 2024 CHecked BY CB17 BOX BEAM DETAILS Route Bildoe ID. Country of S1800027N Boldoe ID. Date Division of Structural Design Date: JULY 2024 CHecked BY CB17 BOX BEAM DETAILS Route 031B00027N EDMONSON Date Division of Structural Design Date: JULY 2024 CHecked BY CB17 BOX BEAM DETAILS Route 031B00027N EDMONSON Date Division of Structural Design Date: JULY 2024 CHecked BY CB17 BOX BEAM DETAILS Route 031B00027N EDMONSON Date Detailed BY: M. Bawithawng J. VAN ZEE S.T. ANDARDS CROSSING BIG REEDY CREEK KY 238 SHEET NO. DRawing Number S7 28916								D(E)	#8	2'-0"		
Image: Size of the state of					ه			Bent Mark C	Keintorce	ement		
a C1(e) Bar C3(e) #4 3'-9" 1'-1 %" C1(e) Bar C3(e) #5 11 %" 1'-1 %" C4(e) #4 3'-9" 1'-6 ¼" C4(e) #4 3'-9" 1'-6 ¼" C4(e) #4 3'-9" 1'-6 ¼" C4(e) #4 3'-9" 1'-6 ¼" C4(e) #4 3'-9" 1'-6 ¼" C4(e) #4 3'-9" 1'-6 ¼" C4(e) #4 3'-9" 1'-6 ¼" C4(e) #4 3'-9" 1'-6 ¼" C4(e) #4 3'-9" 1'-6 ¼" C4(e) #4 3'-9" 1'-6 ¼" C4(e) #4 3'-9" 1'-6 ¼" C4(e) #4 3'-9" 1'-6 ¼" DESIGNED BY J. VAN ZEE S.T. ANDARDS CROSSING DETAILED BY: M. BAWITHAWNG J. VAN ZEE BIG REEDY CREEK KY 238 DETAILED BY: M. BAWITHAWNG J. VAN ZEE BIG REEDY CREEK KY 238 S7 28916					<u>♥</u> []			C1(e)	#5 3'-9"	6"		
Date PREPARED BY Date: JULY 2024 CHECKED BY Date Date: JULY 2024 CHECKED BY Date Division of Date Date: JULY 2024 Designed By: J. VAN ZEE Structural Design JULY 2024 Detailed By: M. Bawiithawing JULY 2024 Detailed By: Date: JULY 2024 Detailed By: J. VAN ZEE Big REEDY CREEK KY 238 Structural Design JULY 2024						$\frac{a}{(1(a) Rar}$			#4 3'-9" #5 11 ³ ⁄	$\frac{1}{1-1} \frac{1}{3/2}$		
DATE PREPARED BY Date: JULY 2024 CHECKED BY CB17 BOX BEAM DETAILS ROUTE BRIDGE ID. COUNTY OF Date: JULY 2024 Date: JULY 2024 CHECKED BY CHECKED BY CHECKED BY CHECKED BY Date: JULY 2024 Date: JULY 2024 Date: JULY 2024 Date: JULY 2024 CHECKED BY CHECKED BY CHECKED BY Date: JULY 2024 Date: JULY 2024 Date: JULY 2024 CHECKED BY CHECKED BY CHECKED BY Date: JULY 2024 Date: JULY 2024 Date: JULY 2024 CHECKED BY CHECKED								C4(e)	#4 3'-9"	1'-6 1/4	·	
CROSSING KY 238 SHEET NO. Structural Design DETAILED BY: M. BAWITHAWNG J. VAN ZEE DATE PLOTTED: 23-OCT-2024		DATE	Division of	DATE: JULY 2024 DESIGNED BY: J. VAN ZEE	CHECKED BY	CB17 BOX BEAM DETA	ILS		BRIDGE ID. 81B00027N	EDMC	NTY OF DNSON	
		· 23-007 00	- Structural Design	DETAILED BY: M. BAWITHAWNG	J. VAN ZEE	BIG REEDY CREEK		KY 238	SHEET NO. S7	DRAWING	∍ NUMBER 916	



		2 ¹ ⁄₂"cl	
			5 1/2"
		 1½"cl. 	
			1'-10"
	•		
			5
3 spa. @ 2"	4"	1 1/4 "cl.	<u> </u>
		•	

DATE		DATE: JULY 2024	CHECKED BY	С
	Division of	DESIGNED BY: J. VAN ZEE	S.T. ANDARDS	
	Structural Design	DETAILED BY: M. BAWITHAWNG	J. VAN ZEE	



	DATE	PREPARED BY	DATE: JULY 2024	CHECKED BY	
		Division of	· · · · · · · · · · · · · · · · · · ·		
			DESIGNED BY: J. VAN ZEE	S.T. ANDARDS	
		Structural Design	DETAILED BY: M. BAWITHAWNG	J. VAN ZEE	
TTED	- 00 OCT 0004		- NEdmonoon\024D00027NI\Einal Eilaa far Latting	and Conto/20016 dam	



DATE	PREPARED BY	DATE: JULY 2024	CHECKED BY	CONSTRUCTION ELEVATION	ROUTE	BRIDGE ID.	
	Division of	DESIGNED BY: J. VAN ZEE	S.T. ANDARDS		KV 238	U31BUUU27N SHEET NO.	EDIVIONSON DRAWING NUMBER
	Structural Design	DETAILED BY: M. BAWITHAWNG	J. VAN ZEE	BIG REEDY CREEK	11 230	S10	28916

FILE NAME: J:\District03\RS & M\Edmonson\031B00027N\Final Files for Letting and Certs\28916.dgn

Note to Resident: The "Maximum Allowable Camber" shown on the beam sheet is the amount of camber, measured prior to casting the deck, above which the beam will

computed dimension "X" is less than that, adjustmants will need to be made to the "X" dimensions on some or all grid lines. Adjustments must meet approval of the

		С	ONSTI	RUCTION	N ELEVA	TIONS			
	L	_eft Fascia			🗆 Bridge		R	ight Fascia	
LUCATION	CONSTR.	TOP OF	DIM.	CONSTR.	TOP OF	DIM.	CONSTR.	TOP OF BEAM	DIM. "X"
SKEW LN AA	99.349			99.560			99.349		
SKEW LN BB	99.349			99.560			99.349		
SKEW LN CC	99.349			99.560			99.349		
SKEW LN DD	99.349			99.560			99.349		
SKEW LN EE	99.349			99.560			99.349		
GRID LN Ø1	0.000			0.000			99.361		
GRID LN Ø2	99.351			99.574			99.371		
GRID LN Ø3	99.364			99.583			99.374		
GRID LN Ø4	99.373			99.585			99.369		
GRID LN Ø5	99.374			99.579			99.357		
GRID LN Ø6	99.367			99.566			99.355		
GRID LN Ø7	99.354			99.568			99.368		
GRID LN Ø8	99.359			99.581			99.379		
GRID LN Ø9	99.371			99.591			99.387		
GRID LN 10	99.382			99.599			99.391		
GRID LN 11	99.388			99.602			99.390		
GRID LN 12	99.391			99.600			99.385		
GRID LN 13	99.389			99.595			99.376		
GRID LN 14	99.383			99.585			99.364		
GRID LN 15	99.373			99.573			99.351		
GRID LN 16	99.361			0.000			0.000		



COMMONWEALTH OF KENTUCKY DEPARTMENT OF HIGHWAYS

REVISION

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USER: Brian.Miller

DATE	PREPARED BY	DATE: JULY 2024	CHECKED BY		ROUTE	BRIDGE ID.	COUNTY OF
	Division of			CONSTRUCTION ELEVATION		031B00027N	EDMONSON
		DESIGNED BY: J. VAN ZEE	S.T. ANDARDS	CROSSING	KY 238	SHEET NO.	DRAWING NUMBER
	Structural Design	DETAILED BY: M. BAWITHAWNG	J. VAN ZEE	BIG REEDY CREEK		S11	28916
DATE PLOTTED: 23-OCT-2024	FILE NAME: J:\District03\RS	& M\Edmonson\031B00027N\Final Files for Lettir	ng and Certs\28916.dgn				



 17
 ₩EST
 M3-4 (24"X12")

 18
 EAST
 M3-2 (24"X12")

 23
 ▲
 M4-9 (30"x24")



Division of Designed by: J. VAN ZEE S.T. ANDARDS Structural Design Detailed by: B. Miller J. VAN ZEE	DATE	PREPARED BY	DATE: JULY 2024	CHECKED BY	
Structural Design DETAILED BY: B. Miller J. VAN ZEE		Division of	DESIGNED BY: J. VAN ZEE	S.T. ANDARDS	
		Structural Design	DETAILED BY: B. Miller	J. VAN ZEE	